

## ACCURACY ASSESSMENT OF DIRECT GEOREFERENCING FOR PHOTOGRAMMETRIC APPLICATIONS ON SMALL UNMANNED AERIAL PLATFORMS

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### ABSTRACT:

Efficient mapping from unmanned aerial platforms cannot rely on aerial triangulation using known ground control points. The cost and time of setting ground control, added to the need for increased overlap between flight lines, severely limits the ability of small VTOL platforms, in particular, to handle mapping-grade missions of all but the very smallest survey areas. Applanix has brought its experience in manned photogrammetry applications to this challenge, setting out the requirements for increasing the efficiency of mapping operations from small UAVs, using survey-grade GNSS-Inertial technology to accomplish direct georeferencing of the platform and/or the imaging payload. The Direct Mapping Solution for Unmanned Aerial Vehicles (DMS-UAV) is a complete and ready-to-integrate OEM solution for Direct Georeferencing (DG) on unmanned aerial platforms. Designed as a solution for systems integrators to create mapping payloads for UAVs of all types and sizes, the DMS produces directly georeferenced products for any imaging payload (visual, LiDAR, infrared, multispectral imaging, even video). Additionally, DMS addresses the airframe's requirements for high-accuracy position and orientation for such tasks as precision RTK landing and Precision Orientation for Air Data Systems (ADS), Guidance and Control.

This paper presents results using a DMS comprised of an Applanix APX-15 UAV with a Sony a7R camera to produce highly accurate orthorectified imagery without Ground Control Points on a Microdrones md4-1000 platform conducted by Applanix and Avyon. APX-15 UAV is a single-board, small-form-factor GNSS-Inertial system designed for use on small, lightweight platforms. The Sony a7R is a prosumer digital RGB camera sensor, with a 36MP, 4.9-micron CCD producing images at 7360 columns by 4912 rows. It was configured with a 50mm AF-S Nikkor f/1.8 lens and subsequently with a 35mm Zeiss Sonnar T\* FE F2.8 lens. Both the camera/lens combinations and the APX-15 were mounted to a Microdrones md4-1000 quad-rotor VTOL UAV. The Sony A7R and each lens combination were focused and calibrated terrestrially using the Applanix camera calibration facility, and then integrated with the APX-15 GNSS-Inertial system using a custom mount specifically designed for UAV applications. The mount is constructed in such a way as to maintain the stability of both the interior orientation and IMU boresight calibration over shock and vibration, thus turning the Sony A7R into a metric imaging solution.

In July and August 2015, Applanix and Avyon carried out a series of test flights of this system. The goal of these test flights was to assess the performance of DMS APX-15 direct georeferencing system under various scenarios. Furthermore, an examination of how DMS APX-15 can be used to produce accurate map products without the use of ground control points and with reduced sidelap was also carried out. Reducing the side lap for survey missions performed by small UAVs can significantly increase the mapping productivity of these platforms.

The area mapped during the first flight campaign was a 250m x 300m block and a 775m long railway corridor in a rural setting in Ontario, Canada. The second area mapped was a 450m long corridor over a dam known as Fryer Dam (over Richelieu River in Quebec, Canada). Several ground control points were distributed within both test areas.

The flight over the block area included 8 North-South lines and 1 cross strip flown at 80m AGL, resulting in a ~1cm GSD. The flight over the railway corridor included 2 North-South lines also flown at 80m AGL. Similarly, the flight over the dam corridor included 2 North-South lines flown at 50m AGL. The focus of this paper was to analyse the results obtained from the two corridors.

Test results from both areas were processed using Direct Georeferencing techniques, and then compared for accuracy against the known positions of ground control points in each test area. The GNSS-Inertial data collected by the APX-15 was post-processed in Single Base mode, using a base station located in the project area via POSPac UAV. For the block and railway corridor, the base-

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station's position was precisely determined by processing a 12-hour session using the CSRS-PPP Post Processing service. Similarly, for the flight over Fryer Dam, the base-station's position was also precisely determined by processing a 4-hour session using the CSRS-PPP Post Processing service. POSPac UAV's camera calibration and quality control (CalQC) module was used to refine the camera interior orientation parameters using an Integrated Sensor Orientation (ISO) approach. POSPac UAV was also used to generate the Exterior Orientation parameters for images collected during the test flight.

The Inpho photogrammetric software package was used to develop the final map products for both corridors under various scenarios. The imagery was first imported into an Inpho project, with updated focal length, principal point offsets and Exterior Orientation parameters. First, a Digital Terrain/Surface Model (DTM/DSM) was extracted from the stereo imagery, following which the raw images were orthorectified to produce an orthomosaic product.

## 1. INTRODUCTION

### 1.1 Direct Georeferencing and Integrated Sensor Orientation

For aerial mapping applications, the Exterior Orientation (EO) of each image is required to produce map products such as orthomosaics and terrain models. There are two ways of computing the EO. In the first method, measurements from a differential GNSS system integrated with an Inertial Measurement Unit (IMU) such as the APX-15 (Figure 1) can be used for the direct determination of the EO parameters. This technique is known as Direct Georeferencing (DG). The second method involves using traditional Aerial Triangulation (AT) on a block of images along with a good distribution of Ground Control Points (GCPs) to solve for the EO. The benefits of Direct Georeferencing versus traditional AT for photogrammetric applications have been well studied (Hutton et al., 2005). However, the accuracy achieved by a DG system is limited by the accuracy of the camera Interior Orientation (IO), IMU-camera misalignment (boresight) angles and datum errors. These errors can be mitigated by using the direct EO in a traditional block bundle adjustment. This technique is referred to as Integrated Sensor Orientation or ISO (Ip et al, 2004).



Figure 1: Applanix APX-15 UAV Single Board GNSS-Aided Inertial solution for Direct Georeferencing on UAVs

This paper investigates how well the DG and ISO approach applies to mapping from small Unmanned Aerial Vehicles (UAVs) for corridor mapping by eliminating the extensive distribution of GCPs and flying with minimal sidelap for improved efficiencies and reduced cost.

## 2. PERFORMANCE TEST RESULTS

### 2.1 Test Overview

On July 22, 2015, Avyon and Applanix conducted a series of test flights with the Microdrones md4-1000 quadcopter, equipped with Sony a7R camera (50mm AF-S Nikkor f/1.8 lens), and rigidly mounted to an APX-15 GNSS-Inertial system (Figure 2). The timing of the Sony a7R sensor was previously characterized and calibrated to ensure that the imagery is

accurately time stamped at the mid-exposure pulse of the camera.

The first test area consisted of a rectangular block, approximately 300m x 250m in size as well as a 775m x 90m long railway corridor. A network of approximately 40 GCPs was established within this test area, and surveyed (Figure 3). The test flight over the block was conducted with 8 flight lines in the North-East to South-West direction (adjacent lines flown in opposite directions), and one perpendicular cross line. The flying altitude was 80m AGL resulting in a GSD of ~7.8 mm. A GNSS base station was established within the test area, and the raw data logged for the duration of the campaign.

The railway corridor was captured by flying two opposing North-East and South-West strips. The sidelap between the two strips was ~50% while the endlap within each strip was ~80%. Similar to the block, the flying altitude was 80m AGL resulting in a GSD of ~7.8mm.

On August 19, 2015, Avyon and Applanix conducted a second flight test over Fryer Dam with the Microdrones md4-1000 quadcopter, equipped with Sony a7R camera (35mm Zeiss Sonnar T\* FE F2.8 lens) also rigidly mounted to an APX-15 GNSS-Inertial system. The Fryer Dam is a water control dam located North of Lake Champlain in Quebec, Canada. The goal of this test was to investigate the feasibility of using a UAV and DG technology to map dams and similar structures for inspection and inventory purposes. A dam over water represents a particularly challenging problem for traditional AT based solutions since point matching on water is not possible. This test area consisted of a 450m x 100m corridor captured by flying two opposing North-East and South-West strips. The endlap was ~85% while the sidelap was ~30%. The flying altitude was approximately 50m AGL resulting in a GSD of ~7 mm. A total of 5 GCPs were established in the survey area (Figure 4). It was not possible to establish GCPs on the dam structure itself. A GNSS base station was also setup near the test area, and the raw data was logged for the duration of the aerial survey.

For all missions, the relevant flight plans were created and uploaded into the UAV's flight management system. After manual take-off, the UAV was switched into automatic waypoint mode, following which it proceeded to fly the survey lines autonomously.

The captured images and APX-15 UAV raw sensor data for each mission was subsequently downloaded from the payload sensor for processing and analysis.



Figure 2: Microdrones md4-1000 VTOL UAV with the APX-15 UAV and dSLR camera



Figure 3: GCP distribution for the block and railway corridor viewed in Google Maps

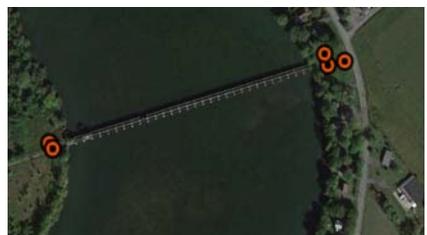


Figure 4: GCP distribution for Fryer Dam viewed in Google Maps

## 2.2 Railway Corridor

The Sony a7R camera and 50mm Nikkor lens combination were terrestrially calibrated using Applanix' in-house camera calibration facility for approximate focal length, principal point and lens distortion parameters using a process certified by the U.S. Geological Survey (USGS). The data collected over the block was previously processed and analysed (Mian et al., 2015) and was not further investigated in this paper.

The GNSS-Inertial data collected by the APX-15 UAV over the railway corridor was post-processed in POSPac UAV Single Base mode, using a base station within the project area (Figure 5 & 6). The position of this base station was precisely determined by processing a long 12-hour static observation session using the CSRS-PPP service provided by Natural Resources Canada.

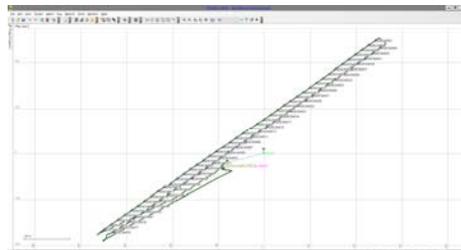


Figure 5: APX-15 UAV trajectory with photo centres - Railway Corridor



Figure 6: POSPac UAV trajectory RMS estimate – Railway Corridor

The mission data was processed through the Applanix Calibration and Quality Control application (CalQC) - bundle adjustment software. First, tie points were extracted using the a-priori EO from POSPac UAV and the approximate camera interior orientation from the terrestrial calibration. The tie-points and a-priori EO were then run in a bundle adjustment where the IMU-camera misalignment (boresight) angles were estimated and the focal length and principal point offsets refined from their approximate values using the ISO approach. Lens distortion parameters were held fixed. A single 3-dimensional control point was used as part of the bundle adjustment (Figure 7) to perform quality control on the focal length. The refined camera parameters and boresight estimates were subsequently used to generate the final map products.

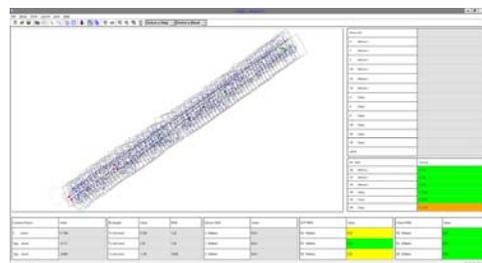


Figure 7: CalQC bundle adjustment project – Railway Corridor

## 2.3 Accuracy Assessment - Railway Corridor

A map view of the orthorectified imagery is shown below (Figure 8).



Figure 8: GCPs overlaid on orthophotos displayed in Global Mapper software

### 2.3.1 Map Accuracy – 2 Strip ISO, 50% sidelap

The Inpho photogrammetric software package was used to develop ortho-images from the Sony a7R imagery. The photos were imported into an Inpho project (Figure 9), with updated focal length, principal point offsets and estimated boresight angles resulting from the CalQC bundle adjustment. Inpho's Match-AT module was used to update the EO parameters using 1 GCP.

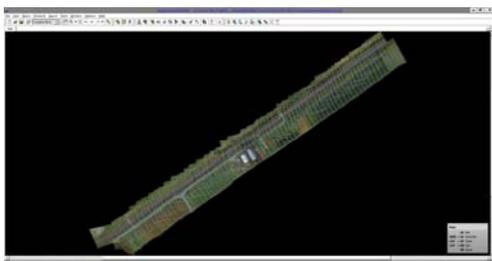


Figure 9: 2 Strip Inpho Project

First, a 20cm Digital Terrain Model (DTM) was extracted using Inpho MATCH-T DSM version 6.1. Using this DTM, the raw images were then orthorectified at a GSD of 1cm using Inpho OrthoMaster version 6.1.

The map accuracy was evaluated by comparing the GCP positions in the orthomosaic and DTM products against their surveyed positions.

The estimated map accuracy values are summarized below. A more detailed list of results is presented in Table A1 in Appendix A.

	dE	dN	dH
Number of Points	12	12	12
Mean Error	-0.010	-0.004	-0.003
Standard Deviation (m)	0.048	0.037	0.079
RMSE (m)	0.073	0.055	0.118
RMSEr (m)	0.092	SQRT(RMSEx2 + RMSEy2)	
NSSDA Horizontal Accuracy (ACCr) at 95% Confidence Level	0.159 RMSEr × 1.7308		
NSSDA Vertical Accuracy (ACCz) at 95% Confidence Level	0.231 RMSEz × 1.9600		

Table 1: 2 Strip ISO Accuracy Results, 50% sidelap - Railway Corridor

### 2.3.2 Map Accuracy – Single Strip ISO

The data collected over the railway corridor was re-processed, this time only considering a single strip. A new Inpho project (Figure 10) was created with the updated focal length, principal point offsets and estimated boresight angles obtained from

running the CalQC bundle adjustment on the block dataset (Mian et al., 2015). The Match-AT module was used to update the EO parameters using 1 GCP. A 20cm DTM was first extracted and the raw images were in turn orthorectified at a GSD of 1cm. The estimated map accuracy values are summarized below. A more detailed list of results is presented in Table A2 in Appendix A.

	dE	dN	dH
Number of Points	12	12	12
Mean Error	0.013	0.003	-0.056
Standard Deviation (m)	0.036	0.037	0.162
RMSE (m)	0.057	0.055	0.255
RMSEr (m)	0.079	SQRT(RMSEx2 + RMSEy2)	
NSSDA Horizontal Accuracy (ACCr) at 95% Confidence Level	0.137 RMSEr × 1.7308		
NSSDA Vertical Accuracy (ACCz) at 95% Confidence Level	0.499 RMSEz × 1.9600		

Table 2: Single Strip ISO Accuracy Results - Railway Corridor

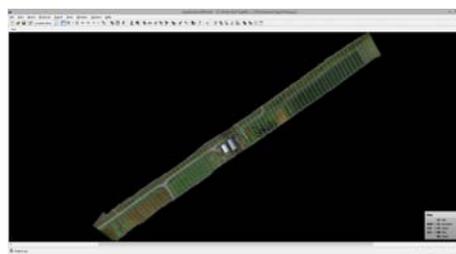


Figure 10: Single Strip Inpho Project

### 2.3.3 Map Accuracy – Single Strip DG

In this processing run, again only a single strip over the railway corridor was considered. The updated focal length, principal point offsets and estimated boresight angles resulting from the CalQC bundle adjustment run on the block dataset (Mian et al., 2015) were used to setup the Inpho project. However, this time around the apriori EO was not updated by running the Match-AT module. Similar to the previously mentioned single strip corridor run, a 20cm DTM was extracted from the stereo imagery. The raw images were also orthorectified at a GSD of 1cm. The estimated map accuracy values are summarized below. A more detailed list of results is presented in Table A3 in Appendix A.

	dE	dN	dH
Number of Points	12	12	12
Mean Error	0.001	-0.044	-0.109
Standard Deviation (m)	0.045	0.044	0.238
RMSE (m)	0.067	0.095	0.391
RMSEr (m)	0.116	SQRT(RMSEx2 + RMSEy2)	
NSSDA Horizontal Accuracy (ACCr) at 95% Confidence Level	0.201 RMSEr × 1.7308		
NSSDA Vertical Accuracy (ACCz) at 95% Confidence Level	0.767 RMSEz × 1.9600		

Table 3: DG Accuracy Results - Railway Corridor

### 2.3.4 Observations and Discussion of Results

A comparison of the horizontal and vertical RMS accuracy results for the three test cases is given in the following table:

	2 Strip, 50% sidelap, ISO	Single Strip, ISO	Single Strip, DG
RMS Horizontal Accuracy (m)	0.092	0.079	0.116
RMS Vertical Accuracy (m)	0.118	0.255	0.391

Table 4: Summary of Accuracy Results - Railway Corridor

The following observations can be made:

- The accuracy of the APX-15 UAV and stability of the Sony a7R camera with 50mm lens on the md4-1000 are sufficient to produce map products to an accuracy of better than 12cm horizontal RMS and 40cm

vertical RMS along a single strip corridor at 80m HAG, without the use of *any* GCPs or AT.

- The accuracy of the map products in the single strip corridor can be improved by using 1 GCP and ISO to a level better than 8cm horizontal RMS and 26cm vertical RMS.
- Adding a second strip to the corridor with 50% sidelap and using 1 GCP with ISO dramatically improved the vertical accuracy to better than 12cm RMS, but degraded the horizontal accuracy to 9cm RMS.

These levels of absolute accuracy are more than adequate for many types of corridor mapping applications such as rail side inventory, damage assessment, wildlife management and more.

The improvements observed using ISO are consistent with the theory and analysis performed over the past decade with cameras on manned aircraft. While the horizontal accuracy was slightly degraded by using two strips, this is most likely explained by the quality of the point matching. For this analysis, an automatic approach was taken without any detailed analysis or filtering of the tie points. As discussed in Ip et al, 2004, having too many tie points can degrade the results of ISO, especially if their quality is not “the best”.

## 2.4 Fryer Dam

The Sony a7R camera and a 35mm Zeiss Sonnar lens combination were previously terrestrially calibrated using Applanix’ in-house camera calibration facility for approximate focal length, principal point and lens distortion parameters.

The GNSS-Inertial data collected by the APX-15 UAV over the dam corridor was post-processed in POSPac UAV Single Base mode, using a base station within the project area (Figure 11 & 12). The position of this base station was precisely determined by processing a 4-hour static observation session using the CSRS-PPP service.

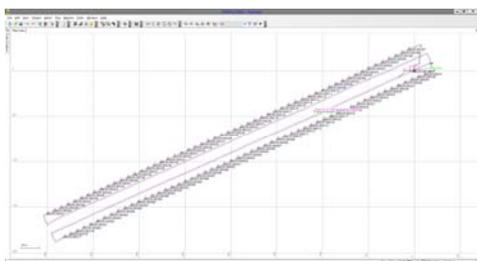


Figure 11: APX-15 UAV trajectory with photo centres - Fryer Dam

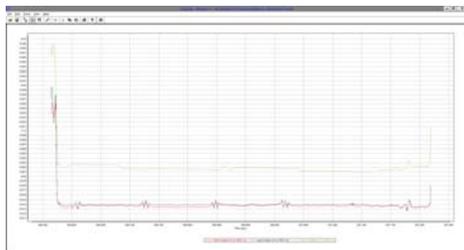


Figure 12: POSPac UAV trajectory RMS estimate- Fryer Dam

Similar to the above runs, the mission data was processed through the Applanix Calibration and Quality Control application (CalQC) - bundle adjustment software. First, tie points were extracted using the a-priori EO from POSPac UAV and the approximate camera interior orientation obtained from the terrestrial calibration. The tie-points and a-priori EO were then run in a bundle adjustment where the boresight angles were estimated and the focal length and principal point offsets refined from their approximate values. Lens distortion parameters were held fixed. A single 3-dimensional GCP was used as part of the bundle adjustment (Figure 13) to perform quality control on the focal length. The refined EO from the adjustment process was then used to generate the final map products.

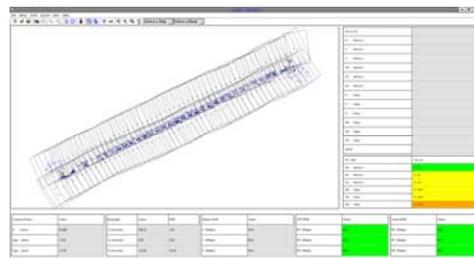


Figure 13: CalQC bundle adjustment project – Fryer Dam

## 2.5 Accuracy Assessment – Fryer Dam

A map view of the orthorectified imagery is shown below (Figure 14).

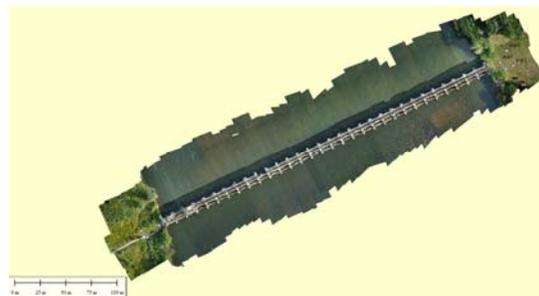


Figure 14: GCPs overlaid on orthophotos displayed in Global Mapper software

### 2.5.1 Map Accuracy – Fryer Dam

The Inpho photogrammetric software package was used to develop ortho-images from the Sony a7R imagery and 35mm Zeiss lens. The photos were imported into an Inpho project, with the updated EO, focal length, principal point offsets and estimated boresight angles resulting from the CalQC bundle adjustment.

First, a 7cm Digital Surface Model (DSM) was extracted and, in turn, used to orthorectify the images at a GSD of 1cm. The estimated map accuracy values are summarized below. A more detailed list of results is presented in Table A4 in Appendix A.

	dE	dN	dH
Number of Points	5	5	5
Mean Error	0.031	-0.009	-0.023
Standard Deviation (m)	0.017	0.014	0.101
RMSE (m)	0.034	0.015	0.093
RMSEr (m)	0.038	SQRT(RMSE <sub>x</sub> <sup>2</sup> + RMSE <sub>y</sub> <sup>2</sup> )	
NSSDA Horizontal Accuracy (ACC <sub>r</sub> ) at 95% Confidence Level	0.065	RMSE <sub>r</sub> × 1.7308	
NSSDA Vertical Accuracy (ACC <sub>z</sub> ) at 95% Confidence Level	0.182	RMSE <sub>z</sub> × 1.9600	

Table 5: 2 Strip Accuracy Results, ISO, 30% sidelap – Fryer Dam

### 2.5.2 Observations and Discussion of Results

For the dam project, the horizontal accuracy was better than 4cm RMS, and the vertical accuracy better than 10cm RMS against Check Points, even though the sidelap between strips was only 30%. The improved accuracy over the Railway Corridor can likely be explained due to the improved base to height ratio offered by the 35mm lens and lower flying height. While the number of Check Points was only 5, and these were distributed on the ground at each end of the dam, the expectation is the accuracy should be consistent throughout the project just as it was demonstrated with the Railway corridor. Of course, the best test would be to measure some Check Points on the dam structure itself, which unfortunately was not possible.

### 3. CONCLUSIONS

Direct Georeferencing and Integrated Sensor Orientation eliminate the need for dense GCPs and allow the capture of image data with minimal sidelap to increase data acquisition and processing efficiencies. The processing time required to create map products compared to traditional AT techniques is greatly reduced thereby increasing productivity. For example, the end to end processing time for the Railway Corridor for the single strip run was approximately 30 minutes. The tests outlined in this paper demonstrate the feasibility of using DG in corridor projects based upon a prosumer camera such as the Sony a7R sensor to generate highly efficient, accurate and cost effective Directly Georeferenced map products from a payload small enough to fit into a small unmanned aerial platform.

### 4. FUTURE WORK

Further analysis will be done on Directly Georeferenced payload sensors on board fixed wing UAV platforms to investigate the effect of refining additional IO parameters in the relative bundle adjustment such as lens distortion.

### 5. ACKNOWLEDGEMENTS

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### APPENDIX A

Point ID	Survey Check Point Values			Map-derived values			Residuals (Errors)		
	E	N	H	E	N	H	dE	dN	dH
gcp	546578.461	5004027.714	26.767	546578.518	5004027.714	26.845	-0.06	-0.03	-0.08
gcq2	546682.536	5004095.026	23.511	546682.467	5004095.010	23.560	0.07	0.02	-0.05
gcq3	546677.279	5004102.613	23.389	546677.209	5004102.600	23.385	0.07	0.01	0.00
gcq4	546838.543	5004221.499	22.925	546838.586	5004221.502	22.982	-0.04	0.00	-0.06
gcq5	546843.109	5004215.924	22.899	546843.173	5004215.916	22.809	-0.06	0.01	0.09
gcq7	546725.198	5004138.812	23.007	546725.175	5004138.800	22.974	0.02	0.01	0.03
gcq11	546617.703	5004059.723	26.154	546617.761	5004059.762	26.340	-0.06	-0.04	-0.19
gcq12	546495.768	5003981.554	27.583	546495.792	5003981.578	27.541	-0.02	-0.02	0.04
gcq13	546502.329	5003966.727	26.133	546502.327	5003966.730	26.035	0.00	0.00	0.10
gcq15	546380.884	5003884.403	24.141	546380.929	5003884.456	24.105	-0.05	-0.05	0.04
gcq16	546372.547	5003896.365	24.231	546372.569	5003896.403	24.198	-0.02	-0.04	0.03
gcq17	546299.273	5003828.631	26.101	546299.240	5003828.545	26.098	0.03	0.09	0.00
Number of Points							12	12	12
Mean Error							-0.010	-0.004	-0.003
Standard Deviation (m)							0.048	0.037	0.079
RMSE (m)							0.073	0.055	0.118
RMSEr (m)							0.092	SQRT(RMSE <sub>x</sub> <sup>2</sup> + RMSE <sub>y</sub> <sup>2</sup> )	
NSSDA Horizontal Accuracy (ACC <sub>r</sub> ) at 95% Confidence Level							0.159	RMSE <sub>r</sub> × 1.7308	
NSSDA Vertical Accuracy (ACC <sub>z</sub> ) at 95% Confidence Level							0.231	RMSE <sub>z</sub> × 1.9600	

Table A1: Map Accuracy for the 2 Strip ISO, 50% sidelap - Railway Corridor

Point ID	Survey Check Point Values			Map-derived values			Residuals (Errors)		
	E	N	H	E	N	H	dE	dN	dH
gcp	546578.461	5004027.714	26.767	546578.506	5004027.684	26.954	-0.05	0.03	-0.19
gcq2	546682.536	5004095.026	23.511	546682.498	5004094.991	23.362	0.04	0.03	0.15
gcq3	546677.279	5004102.613	23.389	546677.214	5004102.592	23.186	0.06	0.02	0.20
gcq4	546838.543	5004221.499	22.925	546838.495	5004221.472	22.967	0.05	0.03	-0.04
gcq5	546843.109	5004215.924	22.899	546843.066	5004215.903	22.998	0.04	0.02	-0.10
gcq7	546725.198	5004138.812	23.007	546725.207	5004138.816	23.180	-0.01	0.00	-0.17
gcq11	546617.703	5004059.723	26.154	546617.735	5004059.652	26.395	-0.03	0.07	-0.24
gcq12	546495.768	5003981.554	27.583	546495.737	5003981.591	27.566	0.03	-0.04	0.02
gcq13	546502.329	5003966.727	26.133	546502.301	5003966.754	26.071	0.03	-0.03	0.06
gcq15	546380.884	5003884.403	24.141	546380.870	5003884.449	24.106	0.01	-0.05	0.03
gcq16	546372.547	5003896.365	24.231	546372.534	5003896.409	24.276	0.01	-0.04	-0.04
gcq17	546299.273	5003828.631	26.101	546299.308	5003828.640	26.446	-0.03	-0.01	-0.35
Number of Points							12	12	12
Mean Error							0.013	0.003	-0.056
Standard Deviation (m)							0.036	0.037	0.162
RMSE (m)							0.057	0.055	0.255
RMSEr (m)							0.079	SQRT(RMSE <sub>x</sub> <sup>2</sup> + RMSE <sub>y</sub> <sup>2</sup> )	
NSSDA Horizontal Accuracy (ACC <sub>r</sub> ) at 95% Confidence Level							0.137	RMSE <sub>r</sub> × 1.7308	
NSSDA Vertical Accuracy (ACC <sub>z</sub> ) at 95% Confidence Level							0.499	RMSE <sub>z</sub> × 1.9600	

Table A2: Map Accuracy for the Single Strip ISO - Railway Corridor

Point ID	Survey Check Point Values			Map-derived values			Residuals (Errors)		
	E	N	H	E	N	H	dE	dN	dH
gcp	546578.461	5004027.714	26.767	546578.437	5004027.717	26.772	0.02	0.00	0.00
gcq2	546682.536	5004095.026	23.511	546682.506	5004095.102	23.314	0.03	-0.08	0.20
gcq3	546677.279	5004102.613	23.389	546677.267	5004102.657	23.585	0.01	-0.04	-0.20
gcq4	546838.543	5004221.499	22.925	546838.577	5004221.562	23.022	-0.03	-0.06	-0.10
gcq5	546843.109	5004215.924	22.899	546843.112	5004215.959	22.969	0.00	-0.04	-0.07
gcq7	546725.198	5004138.812	23.007	546725.108	5004138.916	22.722	0.09	-0.10	0.29
gcq11	546617.703	5004059.723	26.154	546617.738	5004059.755	26.342	-0.04	-0.03	-0.19
gcq12	546495.768	5003981.554	27.583	546495.793	5003981.543	27.730	-0.02	0.01	-0.15
gcq13	546502.329	5003966.727	26.133	546502.323	5003966.718	26.143	0.01	0.01	-0.01
gcq15	546380.884	5003884.403	24.141	546380.845	5003884.433	24.204	0.04	-0.03	-0.06
gcq16	546372.547	5003896.365	24.231	546372.637	5003896.393	24.646	-0.09	-0.03	-0.41
gcq17	546299.273	5003828.631	26.101	546299.274	5003828.767	26.701	0.00	-0.14	-0.60
Number of Points							12	12	12
Mean Error							0.001	-0.044	-0.109
							dE	dN	dH
Standard Deviation (m)							0.045	0.044	0.238
RMSE (m)							0.067	0.095	0.391
RMSEr (m)							0.116	$\sqrt{\text{RMSE}_E^2 + \text{RMSE}_N^2}$	
NSSDA Horizontal Accuracy (ACC <sub>r</sub> ) at 95% Confidence Level							0.201	$\text{RMSE}_E \times 1.7308$	
NSSDA Vertical Accuracy (ACC <sub>z</sub> ) at 95% Confidence Level							0.767	$\text{RMSE}_z \times 1.9600$	

Table A3: Map Accuracy for the Single Strip DG - Railway Corridor

Point ID	Survey Check Point Values			Map-derived values			Residuals (Errors)		
	E	N	H	E	N	H	dE	dN	dH
gcp1	626878.840	5028486.010	30.598	626878.788	5028486.033	30.750	0.05	-0.02	-0.15
gcp2	626872.390	5028500.620	30.823	626872.363	5028500.617	30.919	0.03	0.00	-0.10
gcp3	626865.870	5028489.210	30.503	626865.825	5028489.235	30.430	0.05	-0.03	0.07
gcp14	626521.410	5028317.010	30.162	626521.393	5028317.012	30.173	0.02	0.00	-0.01
gcp15	626515.060	5028323.250	30.135	626515.046	5028323.248	30.062	0.01	0.00	0.07
Number of Points							5	5	5
Mean Error							0.031	-0.009	-0.023
Standard Deviation (m)							0.017	0.014	0.101
RMSE (m)							0.034	0.015	0.093
RMSEr (m)							0.038	$\sqrt{\text{RMSE}_E^2 + \text{RMSE}_N^2}$	
NSSDA Horizontal Accuracy (ACC <sub>r</sub> ) at 95% Confidence Level							0.065	$\text{RMSE}_E \times 1.7308$	
NSSDA Vertical Accuracy (ACC <sub>z</sub> ) at 95% Confidence Level							0.182	$\text{RMSE}_z \times 1.9600$	

Table A4: Map Accuracy for 2 strip ISO, 30% sidelap- Fryer Dam